

An Optimization Procedure for Overhead Gantry Crane Exposed to Buckling and Yield Criteria

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ABSTRACT

The current study presents a general optimization procedure that could be used in designing of various structural applications. To validate the performance of the proposed procedure, a real life application of a custom welded I-Beam gantry crane is selected. The crane is composed of three rectangular plates with the same length and different thicknesses and widths welded together by full penetration welds over the span length to form an I-Beam profile. The thicknesses and widths of plates are to be optimized to have the minimum cross section area while respecting yield, buckling, deflection and fatigue criteria. A mathematical procedure based on Timoshenko beam theory and Crane Manufacturers Association of America (CMAA) in combination with the Genetic Algorithm (GA) is presented, and a Mathcad code is implemented to find the optimal I-Beam cross section dimensions. Nine examples are introduced for 8, 12 and 20 m crane span subjected to 10, 20 and 40-ton capacities. It is noticed that the optimized I-section configurations always show narrow and thick lower flange, wider and thinner upper flange and tall and very thin web. The upper flange local buckling and the lateral buckling limits are achieved for all nine cases, 75% of cases for the web buckling limit, about 33% of cases for the fatigue and yield limits whereas the maximum deflection constraint is never critical. The obtained results were verified using ANSYS Workbench software with a 3D Solid Finite Element model and shown good agreement, which confirms that the proposed procedure is efficient.

Keywords: optimization; I-beam; yield; buckling; design criteria; finite element.

Introduction

The gantry cranes are frequently used for different industrial applications. According to CMAA 74-2010 [1], the cranes in real life engineering are classified into five main classes based on their service capacity: standby, light, moderate, heavy and severe service cranes. The overhead gantry crane type is widely used to serve small or medium duty jobs, like a repair shop, buildings service or in a machine shop. The lightweight crane with high capacity design depicts an essential requirement of the industry. To reach such requirement, a customized I-beam crane is a motivating optimization research. Even though standard I-Beam profiles are available, they are just limited to some standard dimensions, which are usually far from the optimum design. The crane weight, concerning initial standard profiles design, could be reduced up to 10% using an optimized beam [2].

Several researches have been conducted on optimization of customized and standard crane beams with different profiles. Gasaet al.[3], developed a numerical model of flange local stresses under the wheels acting points to determine the final dimension of the I-beam girder. The numerical example of 12.5-ton capacity and 25m span with three different wheel thicknesses demonstrated. The mathematical and FE analysis results compared to show an acceptable error range of 6 to 15 %. Also, they mentioned that the lower flange deflection has a great influence on the final girder dimensions.

Other researchers have worked on optimization of the box profile girders [4, 5 and 6]; they had, in general, the similar procedure of optimization but they used different optimization tools. Their objective was investigating the same concept of weight-strength ratio using theoretical optimization routines backed up by Finite Element (FE) simulation.

Qu et al. [4] proposed a modified Ant Colony Optimization (ACO) algorithm with new local search technique using mutation and applied it to solve nonlinear optimization problems having discrete variables. The developed algorithm of Ant Colony Algorithm with Mutation-based (ACAM) used to determine optimal crane design variables and found to be faster by about 20% compared to the genetic algorithm (GA) and by 11% compared to particle swarm algorithm (PSO). Furthermore, it always gives aglobally optimized solution, while the original ACO algorithm may stick at some local solution and fail to go further.

Zuberiet al. [5], examined the effect of rolling load on welded box cross section-crane girder regarding buckling and compression stresses in the flange. The volume of the girder considered as an objective function subjected to the stress and deflection criteria constraints. The built-in MS-Excel nonlinear optimization solver, called Generalized Reduced Gradient (GRG), employed to give preliminary optimized design variables. The obtained values are then

used as initial inputs to ANSYS code that can handle more accurate stress and deflection calculations for verification purpose and do further optimization if needed.

Kumar et al. [6] conducted research that aims to optimize the weight of Electrical Overhead Travelling (EOT) Crane Bridge girder by adding sufficient stiffeners along the girder plate instead of increasing plate thickness. He used mathematical modeling and Finite Element Analysis to investigate the effect of adding stiffeners and then verify the optimal design experimentally. His work concluded that the plate stability could be increased four times using stiffeners without the need to increase the plate thickness.

Liu et al. [2], carried out a parametric FE study of a doubly trolley box-girder using APDL tool in conjunction with a Matlab code that handles the crane parameters. A three-dimensional girder model subjected to various loading conditions established to predict the limit of load-bearing capacity. Two different optimization algorithms, Arc Length Algorithm (ALA) and Nonlinear Stabilization Algorithm (NLA), used in sequence to overcome the optimization failures. The obtained results of their work shown a significant weight reduction of the girder by 16% compared to the original design.

Few publications about the customized I-beam crane girder subjected to yield and buckling criteria are reported. Therefore, the current paper extends the similar techniques mentioned above to optimize custom I-Beam crane designs. Three rectangular plates having the same length (L) and different thicknesses and widths welded by continuous full penetration welds to form a custom I-Beam crane design, see Fig.3. The live load and the beam span are imposed while each plate thickness and width are considered as design variables that need to be determined to have the minimum weight that respecting the yield, buckling, deflection and fatigue criteria. However, The mathematical calculations based on Cranes Manufacturer Association of America (CMAA) design procedure and the Hybrid Genetic algorithm (GA) are used to find the optimal dimensions of the cross section that satisfy the design constraints. A Mathcad platform is written to handle these calculations. Also, a 3D-solid FE model created; stress analyzed and optimized using ANSYS Workbench software.

Design optimization procedure

Highly sophisticated optimization techniques are needed to achieve an optimal crane design that considers yield, buckling, deflection and fatigue criteria. Such techniques must deal with iterative schemes that require a programming language or a mathematical application such as Mathcad. The general trends of solving such problems in the recent years were emphasizing on carrying out a mathematical solution, an FE solution or a math-FE combined solution. The combined solution conducted in two different ways [5]; the first way is carrying out both types of analysis techniques with the same initial values and takes the most optimal results between them. The second one uses the output results of the mathematical solution as input values of an FE solution. The present study follows the second method. The flowchart Fig.1 illustrates the proposed procedure. It starts with problem formulation, i.e., defines design variables, objective function, etc. Follows that entering the data of crane, which are in our case the span length, the rated load, and the material; then performing the optimization Hybrid Genetic Algorithm (GA) code, the details of which are shown in Fig.2, to give the so called Math-Optimal design variables. The Math-optimal design variables are input as initial variables to the FE Optimization phase using ANSYS Workbench 15 software in which the Response Surface Optimization method is used [7,8].

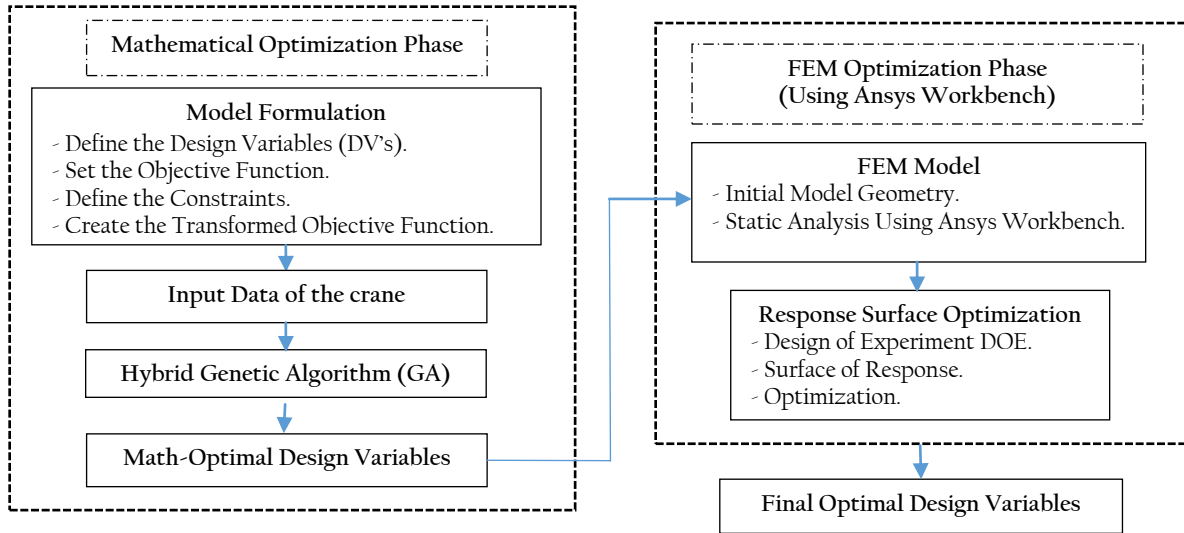


Fig. 1: Proposed design optimization procedure [9]

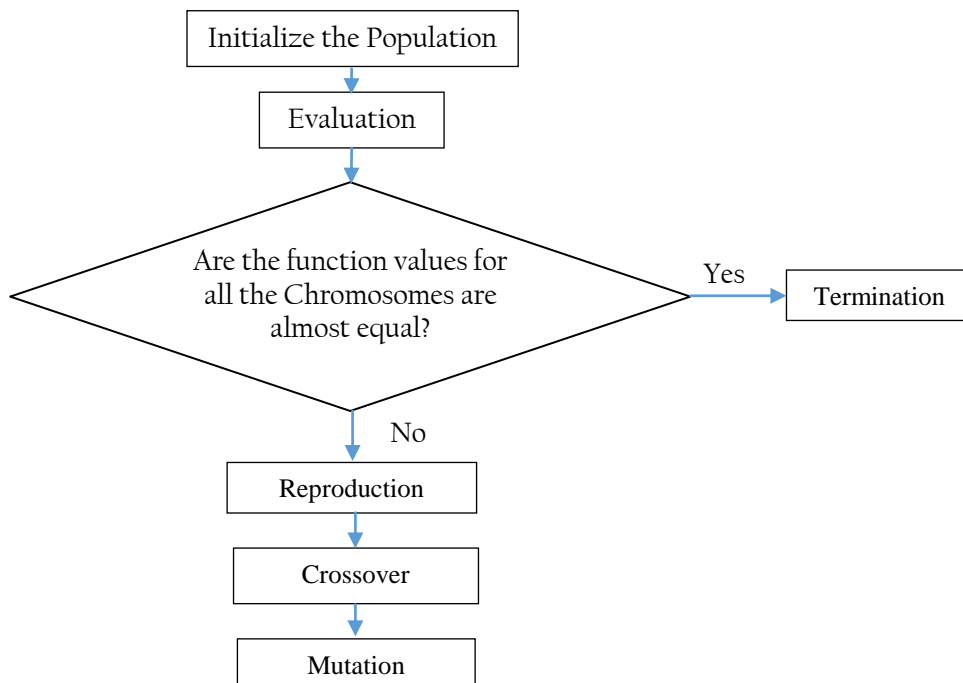


Fig. 2: Hybrid Genetic Algorithm [10]

Problem description

The welded I-Beam crane and the loading conditions are shown in Fig. 3. The beam formed by three plates joined by continuous welds over the beam length. They have the same length but different thicknesses and widths; the dimensions and loading conditions defined as follow:

- | | | |
|---|---------------------------------|-----------------------------------|
| b_1 : lower flange width, | t_1 : lower flange thickness, | b_2 : upper flangewidth, |
| t_2 : upper flangethickness, | h : web height, | t_3 : web thickness, |
| L : beam span, | W_1 : crane weight, | W_2 : live load (Lifting load), |
| x : distance of live load from the left end | | |

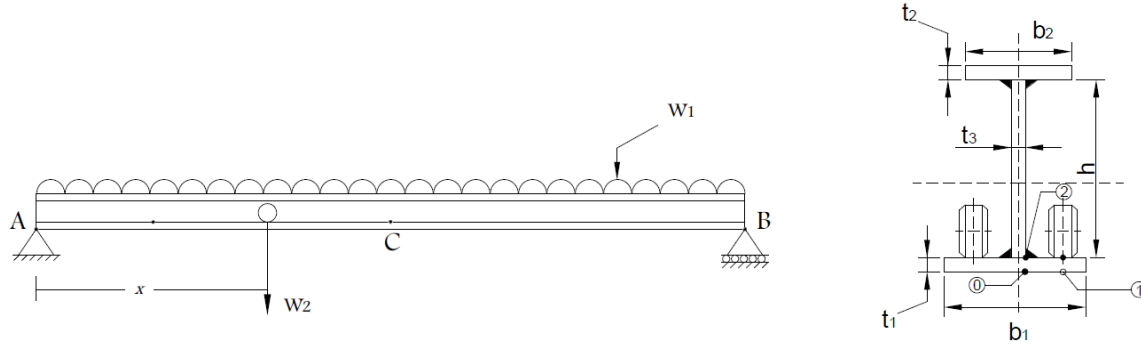


Fig.3: The crane beam dimensions and loading conditions

Objective function

Where the span length is fixed with a constant cross section area of the crane beam, the weight is just proportional to the cross-section area so that the objective function is defined by the cross-section area, as follows.

$$f = b_1 \cdot t_1 + b_2 \cdot t_2 + h \cdot t_3 \tag{1}$$

where the parameters $b_1, t_1 \dots$ etc. are shown in Fig. 3.

Design constraints

The most important criteria of the Crane Manufacturers Association of America specification, known as CMAA74-2010, are considered and summarized as follows

- Tension stress Constraints (due to gravity and live load) :

$$\sigma_{comb_max} - \sigma_{Tallowed} \leq 0 \tag{2}$$

(CMAA 74-3.4.4.1, [1][2])

- Lateral Buckling Constraint:

$$1.9 - f_{Buckling} \leq 0 \tag{3}$$

(Timoshenko Beam Theory, [11][6])

- Local Buckling Constraints:

$$h/t_3 - 260 \leq 0 \tag{4}$$

(AISC 2016 sec. F13, [12][7])

$$b_2/2t_2 - 260/\sqrt{\sigma_y} \leq 0 \tag{5}$$

(CISC handbook p.p5-11, [13][8])

- Deflection Constraint :

$$\delta_v - L/600 \leq 0 \quad (\text{CMAA 74-3.5.5, [1][2]}) \quad (6)$$

- Fatigue Constraint (due to repeated load fluctuation $\Delta W2$ only) :

$$(\Delta\sigma)_{\text{comb max}} - \Delta\sigma_{\text{allowed}} \leq 0 \quad (\text{AISC 2016 sec. F13, [12][7]}) \quad (7)$$

where $\sigma_{\text{comb}} = \sqrt{\sigma_x^2 + \sigma_y^2 - \sigma_x\sigma_y + 3\tau_{xy}^2}$ is the Von-Mises equivalent stress;

- σ_{Tallowed} = Allowable tension stress, according to CMAA 74;
- δ_v = Maximum vertical deflection
- $\Delta\sigma$ = stands for stress range
- σ_Y = Yield strength

f_{Buckling} = Buckling load factor, which means a factor to be multiplied to all applied loads to produce linear buckling of the structure. This factor is given initially by the linear buckling theory, e.g., Timoshenko formulas or by an FE model. It is valid only if the linear buckling stress, which is $\sigma_{cr0} = f_{\text{Buckling}} * |\sigma_{\text{upper flange}}|$, is less than $1/2 * \sigma_Y$; otherwise, it must be modified to take into account the plastic deformation during buckling. The corrected critical stress

calculated using Johnson's empirical formula, $\sigma_{cr} = \sigma_Y \cdot \left[1 - \frac{\sigma_Y}{4 \cdot \sigma_{cr0}} \right]$, [14] and the corrected buckling load factor

is given by $\frac{\sigma_{cr}}{|\sigma_{\text{upper flange}}|}$

Objective function transformation

The exterior point penalty function is used to transform the constrained optimization problem into an unconstrained problem. The general form of the transformed objective function is:

$$F(X, r_h, r_g) = f(X) + r_h \left[\sum_{k=1}^i h_k(X)^2 \right] + r_g \left[\sum_{j=1}^m (\max\{0, g_j(X)\})^2 \right] \quad (8)$$

where X is the vector representing the design variables, h_k is the k th equality constraint if any, g_j is the j th inequality constraint, r_h and r_g are two additional variables called penalty multipliers[15].

Numerical examples

Nine cases defined by three span lengths (8, 12 and 20 m) and three rated loads (10, 20 and 40 tons) are selected as numerical examples. The crane specifications are listed in Table 1. The material used for the crane is 350W structure steel with yielding strength $S_y = 350$ MPa, density $\rho = 7850$ kg/m³, Young's modulus $E = 200$ GPa, shear modulus $G = 77$ GPa and Poisson's ratio $\nu = 0.3$.

Table 1 Crane Specifications according to CMAA 74-2010

Variable	Value/Units
Rated Capacity:	10, 20 or 40 tons
Service Class D:	Heavy Service
Load Class L3:	Normalload = 2/3 of rated load
Cycles Class N2:	Up to 500000 cycles
Span:	8, 12 or 20 m
Trolley Weight:	1 tons
Other equipment Load:	1 tons
Bridge Wheel per rail:	One on each side

The mathematical optimization procedure, described in section 2, programmed using Mathcad Code[16]. Table 2 summarizes the values of GA parameters.

Table 2 Genetic Algorithm Parameters

Parameter	Used Value
Number of Variables:	NV = 6
Population size:	NP = 120
Probability of crossover:	PC = 0.85
Probability of mutation:	PM = 0.05
Mutation Parameter:	BM = 5
Maximum generation number:	GMAX = 300

Finite element model

The Fig. 4 shows a 3D drawing of an I-Beam crane, the Fig. 5 (a) shows the overall view of a 3D-solid FE model of the crane created in ANSYS Workbench © 15, and the Fig. 5 (b) shows a local zoom around the contact region between the lower flange and the wheels. The lower edges at ends are vertically supported, and the loads to be considered are composed of the distributed gravity load W1 (weight of the beam), and the concentrated load W2 applied on the wheels, W2 being the combination of the lifted load, the weights of trolley and hoist.

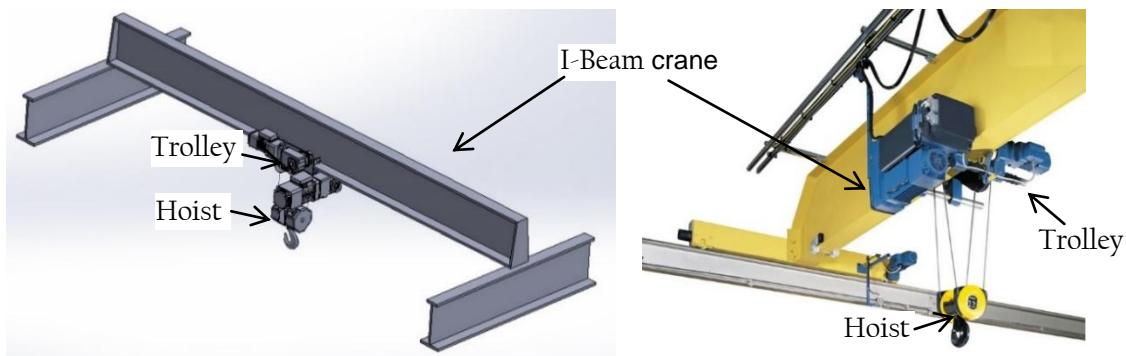


Fig. 4. Three-dimensional images of the crane

All loads are adjusted by factors according to CMAA 74 Specifications. The rated load plus gravity are applied when considering the yield and buckling constraints, inequalities (2) to (5), while the normalload fluctuation, which is just 2/3 of rated load without gravity, is applied when considering the deflection and fatigue constraints, (6) and (7). For the FE model, the Surface Response Optimization method [7, 8], already integrated into ANSYS Workbench, is used. This model contains about 28300 nodes

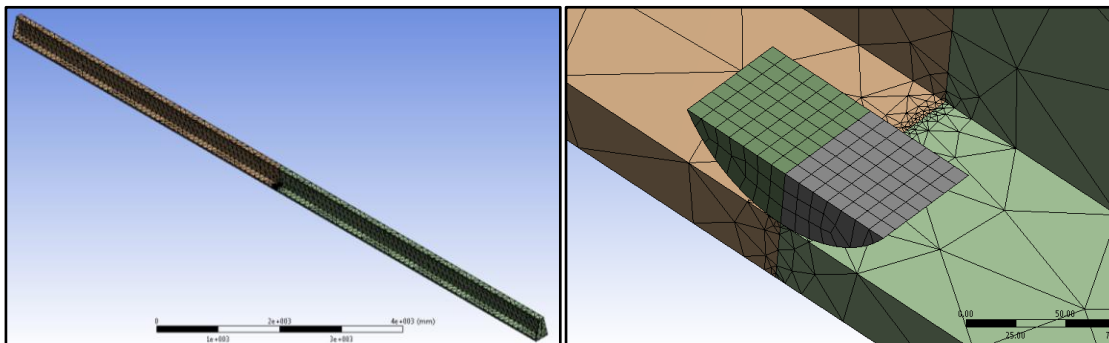


Fig. 5. Finite Element Model of the crane

Numerical results

For reducing calculation time, it needs to input the reasonable lower and upper bound values of each design variable. The bounds used in all 9 cases are shown in Table 3.

Table 3. Lower bound and upper bound of design variables in mm

Variables	t ₁	b ₁	t ₂	b ₂	t ₃	h
Lower bound	2	150	2	150	3	250
Upper bound	100	600	100	600	100	1675

The optimized design variables for nine cases are presented in the Tables 4.1, 4.2 and 4.3. The Table 4.1 results are for short span cranes with three different rated loads, the Table 4.2's are for intermediate span cranes, and the Table 4.3's are for long span cranes.

Table 4.1 Optimal Design variables and constraint parameters for 8 m cranes

L = 8 m	10 tons		20 tons	40 tons	Bounds
	MATH	FEM			
t ₁ (mm)	27.82	27.98	37.88	52.62	[2, 100]
b ₁ (mm)	150.01	150.0	150.16	150.04	[150, 600]
t ₂ (mm)	6.99	7.43	8.38	9.08	[2, 100]
b ₂ (mm)	194.20	186.1	220.18	252.14	[150, 600]
t ₃ (mm)	3.00	3.00	3.19	4.38	[3, 100]
h (mm)	608.64	650.88	826.46	1137.03	[250, 1675]
Area (m ²)	0.00736	0.00753	0.0102	0.0152	
σ _{com_max} (MPa)	221.9	201.9	224.9	225	<= 225 MPa
F _{Buckling}	1.9	1.96	1.9	1.91	>= 1.9
h/t ₃	202.876	216.96	259.37	259.88	<= 260
b ₂ /2t ₂	13.895	12.524	13.14	13.887	<= 13.898
δ _v (m)	0.0074	0.0063	0.0057	0.0043	<= 0.013 m
(Δσ) _{com_max} (MPa)	166	160.1	165.9	165.2	<= 166 MPa

Table 4.2 Optimal Design variables and constraint parameters for 12 m cranes

L =12 m	10 tons	20 tons	40 tons	Bounds
t ₁ (mm)	27.88	53.52	54.61	[2, 100]
b ₁ (mm)	179.54	150.01	206.13	[150, 600]
t ₂ (mm)	10.34	12.04	13.41	[2, 100]
b ₂ (mm)	287.15	334.55	372.14	[150, 600]
t ₃ (mm)	3.02	3.12	4.21	[3, 100]
h (mm)	785.98	811.78	1094.81	[250, 1675]
Area (m ²)	0.0104	0.0146	0.0209	
σ _{com_max} (MPa)	203.9	188.9	225	<= 225 MPa
F _{Buckling}	1.9	1.9	1.9	>= 1.9
h/t ₃	259.93	259.98	259.98	<= 260
b ₂ /2t ₂	13.884	13.896	13.877	<= 13.898
δ _v (m)	0.0095	0.012	0.0097	<= 0.02 m
(Δσ) _{com_max} (MPa)	163	121.3	164.1	<= 166 MPa

Table 4.3 Optimal Design variables and constraint parameters for 20 m cranes

L = 20 m	10 tons	20 tons	40 tons	Limits
t ₁ (mm)	30.14	92.15	54.41	[2, 100]
b ₁ (mm)	275.88	150.29	347.13	[150, 600]
t ₂ (mm)	15.06	14.98	18.1	[2, 100]
b ₂ (mm)	418.41	416.3	501.7	[150, 600]
t ₃ (mm)	3.29	3.36	4.72	[3, 100]
h (mm)	821.75	872.94	1226.51	[250, 1675]
Area (m ²)	0.0173	.0230	0.034	
σ _{com_max} (MPa)	202.5	199.5	218.2	<= 225 MPa
F _{Buckling}	1.9	1.9	1.9	>= 1.9
h/t ₃	249.96	259.47	259.95	<= 260
b ₂ /2t ₂	13.89	13.891	13.896	<= 13.898
δ _v (m)	0.025	0.032	0.023	<= 0.033 m
(Δσ) _{com_max} (MPa)	157.7	94.61	166	<= 166 MPa

It is noticed that the lateral buckling and the upper flange local buckling limits are reached for nine over 9 cases, the web buckling limit for 6/9 cases, the yield and fatigue limits for 3/9 cases and the deflection constraint is never critical. In addition, the optimized I-section configurations always show narrow and thick lower flange, wider and thinner upper flange and tall and very thin web. The Fig. 6 approximately illustrates the optimum I-Beam cross sectional configuration for a 20 m crane subjected to 20 tons lifted load. The comparison between the custom I-beam configuration as shown in Fig. 6, which has A = 0.023 m², and a doubly symmetrical I-beam (t₁ = t₂ = 39.53 mm, b₁ = b₂ = 307 mm, t₃ = 3.85 mm, h = 996 mm and A = 0.028 m²) shows that the customized I-beam could save almost 18% of the weight. The design parameters given by the Math optimization are then inputted to an FE procedure using ANSYS Workbench 15 with a 3D nonlinearsolid model due to the contact between the wheels and the lower flange. The Surface Response Optimization method in ANSYS Workbench used with considering the same constraints, except the linear buckling constraint, because linear buckling does not work with nonlinear contact models. However, the buckling constraint ($f_{Buckling} \geq 1.9$) replaced by an approximate constraint on the slenderness ratio against lateral buckling to give a comparable buckling load factor. This slenderness ratio is given by $\lambda = L/r_{cy}$ where r_{cy} is the lateral radius of gyration of the effective compression area which is empirically the 2/3 outermost of the compression side of the cross section (see Fig. 6). FE stress calculation with nonlinear contact and optimization procedure is very time consuming;so only one case selected to show FE results, which is the 8 m and 10-ton case. The slenderness ratio constraint for this case is $\lambda \leq 190$.

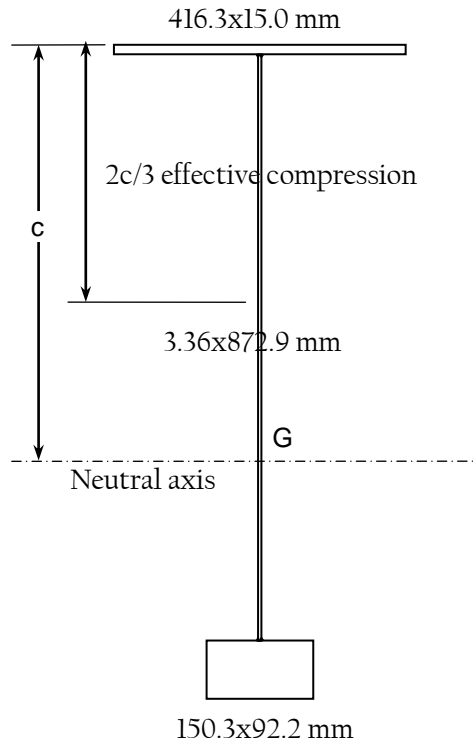


Fig. 6. Optimum configuration of an I-section

The new optimized design parameters given by FE procedure are shown in the FEM column of Table 4.1; they are slightly different but quite close to the Math results. The Fig. 7 reveals that the maximum Von-Mises stress is in the lower flange right under the wheels.

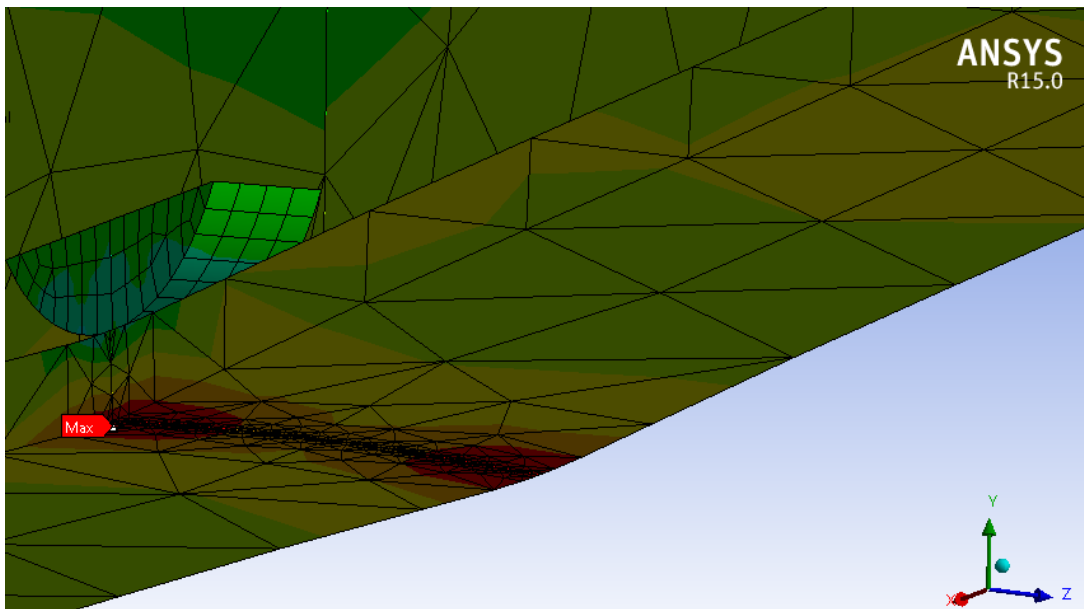


Fig. 7. Location of maximum Von-Mises stress

Conclusion

A Hybrid Genetic Optimization Algorithm (GA) and a Mathematical optimization procedure are programmed in Mathcad and successfully applied to custom welded I-Beam cranes with different spans and rated loads subjected to yield, buckling, deflection and fatigue criteria. It is found that the constraints of general lateral buckling and local buckling of the upper flange are always reached for all cases. The web local buckling constraint is critical for about 66% of cases, the yield and fatigue constraints found critical for 33% of cases and the deflection constraint is not a problem at all. The optimized custom I-section has a configuration of narrow and thick lower flange, thinner and wider upper flange and the web is tall and very thin, which could save about 18% of weight compared to commercial standard I-Beam. FEM optimization using Surface Response method gives comparable results and confirms that the proposed procedure is efficient.

For future works, the FE optimization taking into account nonlinear buckling due to contact or plasticity constitutes a significant challenge. Furthermore, the optimization procedure with multi objective functions such as weight and cost will also be an interesting future work.

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